

# AS-BUILT STORMWATER MANAGEMENT REPORT

FOR

## ROSWELL MANOR

Land Lots 611 & 612 - 1<sup>th</sup> District, 2<sup>nd</sup> Section  
City of Roswell - Fulton County, Georgia



Date: January 10, 2014

Prepared By:



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E303 Project No. 12-3681

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## **SUMMARY**

The subject site is a 16.25 acre tract located in Land Lots 611 and 612 of the 1<sup>st</sup> District, 2<sup>nd</sup> Section of Fulton County in the City of Roswell, Georgia. The property is located north of Old Alabama Road approximately 1,500 feet north of the intersection of Old Alabama Road and Holcomb Bridge Road. The property abuts an office complex

### **Project History**

This site was previously permitted for development in 2007 as a townhome community called Old Alabama Townhomes. The Old Alabama Townhomes project was an 94 unit development with the associated amenities, roadway, utility and stormwater management infrastructure. The project went dormant prior to completion and construction activity ceased after the completion of the infrastructure mentioned above including the stormwater management facilities.

In 2012 development on the site was resumed with a proposed 73 unit fee simple housing layout inclusive of an amenity. This project is known as Roswell Manor and is the subject of this analysis. The current design utilizes the existing infrastructure that was constructed per the approved construction plans for the Old Alabama Townhomes development with only minor modification proposed to the infrastructure to accommodate the new site layout. Given the decreases in site density and impervious area, only maintenance and upkeep was necessary for the existing stormwater management ponds to accommodate the Roswell Manor development.

### **Reduction of Impervious Area**

As previously stated, the housing density was reduced from a 94 unit townhome community to a 73 unit single family development community. Reducing the site density of the project resulted in a net reduction of proposed impervious area. Originally the Old Alabama Townhomes project proposed a total of 366,767 sq. ft. of impervious area. The Roswell Manor project proposes 337,154 sq. ft. of impervious area which is 29,613 sf of impervious area less than what was called for in the original stormwater management design. Ground Cover Take-offs are included in the Runoff Curve Number

Calculations located in the Appendix of this report.

Area take-off calculations were performed using Carlson Hydrology 2014

### Revised Basin Areas and Curve Numbers

The proposed onsite site basins were reassessed to account for minor grading and storm pipe changes that have taken place to accommodate the Roswell Manor development. A summary of the changes to the proposed basins is listed below:

Basin	Design Basin Characteristics	As-Built Basin Characteristics
Basin A	Area: 5.92 Acres CN: 80.2	Area: 5.63 Acres CN: 81.0
Basin A-Bypass	Area: 0.74 Acres CN: 61.5	Area: 0.76 Acres CN: 66.0
Basin B	Area: 8.09 Acres CN: 80.2	Area: 8.51 Acres CN: 80.9
Basin B-Bypass	Area: 0.96 Acres CN: 61.5	Area: 0.97 Acres CN: 61.6
Basin C	Area: 0.15 Acres	Area: 0.07 Acres
Basin D	Area: 0.43 Acres	Area: 0.34 Acres

Please note that the table above only lists the modifications made to the basin characteristics that changed due to the revision in drainage design. The characteristics not listed, such as time of concentration and CN values, were not impacted and the values from the previously approved hydrologic design were used in the as-built model.

### Methodology

This report utilizes the previously approved Hydrology Study for Old Alabama Townhomes by David A. Burre Engineers & Surveyors, Inc., most recently revised March 1, 2007 for predeveloped and as-designed site conditions.

The as-built hydrologic model for this site was built with consideration given to the drainage design modifications for Roswell Manor. The changes to the basin characteristics utilized in this model are previously listed in this report. The as-built ponds were modeled based on field information collected by Engineering303, LLC and the hydrologic model outputs reflect this data.

As-built summaries are provided for each respective pond and are included in this report. Please see report and the appendix for supporting calculations.

The SCS Method was used to analyze the stormwater management for this development.

Hydraflow Hydrographs Extension for AutoCAD Civil 3D 2009, Version 6.066, was used for the hydrologic routing for this study.

## AS-BUILT POND SUMMARIES



### Pond A - As-Built Summary

Pond A				Top of Berm: 1039.7	
Design Storm	As-Designed Release Rates (cfs)	As-Built Release Rates (cfs)	As-Designed Water Surface Elevation (ft)	As-Built Water Surface Elevation (ft)	As-Built Freeboard Provided (ft)
1	2.96	2.93	1037.71	1037.66	2.04
2	4.12	4.05	1037.88	1037.82	1.88
5	8.48	8.03	1038.16	1038.12	1.58
10	13.67	13.04	1038.32	1038.26	1.44
25	19.76	18.80	1038.47	1038.43	1.27
50	23.76	22.59	1038.56	1038.53	1.17
100	27.19	25.20	1038.63	1038.59	1.11

Basin A - Volume Summary						
Design Storm	Water Quality Volume (cf)	Channel Protection Volume	Diameter of Water Quality Orifice (ft)	Elevation of Water Quality Orifice (ft)	Diameter of Channel Protection Orifice (ft)	Elevation of Channel Protection Orifice (ft)
As-Designed	56,105	N/A	N/A	1037.00	N/A	N/A
As-Built	52,944	N/A	N/A	1036.95	N/A	N/A

Note: Pond A is a wet pond with the water quality volume completely contained in the permanent pool thus no draw down orifice is provided for Pond A. The calculated water quality volume required for the entire development is 16,195 cf as per the approved hydrology study by David A. Burre Engineers & Surveyors most recently revised March 1, 2007. The the cumulative as-built water quality volume provided in both Pond A & B is 130,038 cf.

Additionally, Crystal Streams units are provided upstream of each of the outfalls into Pond A. (three units total). Though these units were installed as pre-treatment for Pond A the site will receive added water quality benefit in addition to the treatment received by the wet ponds.

Note: Channel Protection is not provided in Pond A as it was not included in the approved hydrologic design noted above. Thus, channel protection volume and orifice information is not included in the summary table.

Basin A - Flow Summary						
Design Storm	Predeveloped Flow (cfs)	As-Built Flow to Pond (cfs)	As-Built Bypass (cfs)	Allowable Flow* (cfs)	As-Built Pond Flow (cfs)	Flow Reduction (cfs)
1.0	3.7	9.8	0.8	2.9	2.9	0.0
2.0	6.0	12.3	1.1	5.0	4.1	0.9
5.0	11.4	17.5	0.8	10.6	8.0	2.6
10.0	15.9	21.5	2.4	13.6	13.0	0.5
25.0	22.6	26.9	3.2	19.4	18.8	0.6
50.0	28.1	30.9	3.8	24.3	22.6	1.7
100.0	32.0	33.7	4.3	27.7	25.2	2.5

\*Allowable Flow = Predeveloped Flow less As-Built Bypass

Allowable Flow is the basis of comparison for the pond release rates as per the approved hydrology study. The Flow Summary table format was taken from the originally approved Hydrology Study noted above.

# Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Tuesday, Jan 7, 2014

## Pond No. 1 - Detention Pond A

### Pond Data

Contours - User-defined contour areas. Average end area method used for volume calculation. Beginning Elevation = 1036.95 ft

### Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	1036.95	17,000	0	0
0.55	1037.50	17,000	9,350	9,350
1.05	1038.00	17,000	8,500	17,850
1.35	1038.30	17,000	5,100	22,950
2.05	1039.00	20,050	12,968	35,918
3.05	1040.00	21,125	20,588	56,505

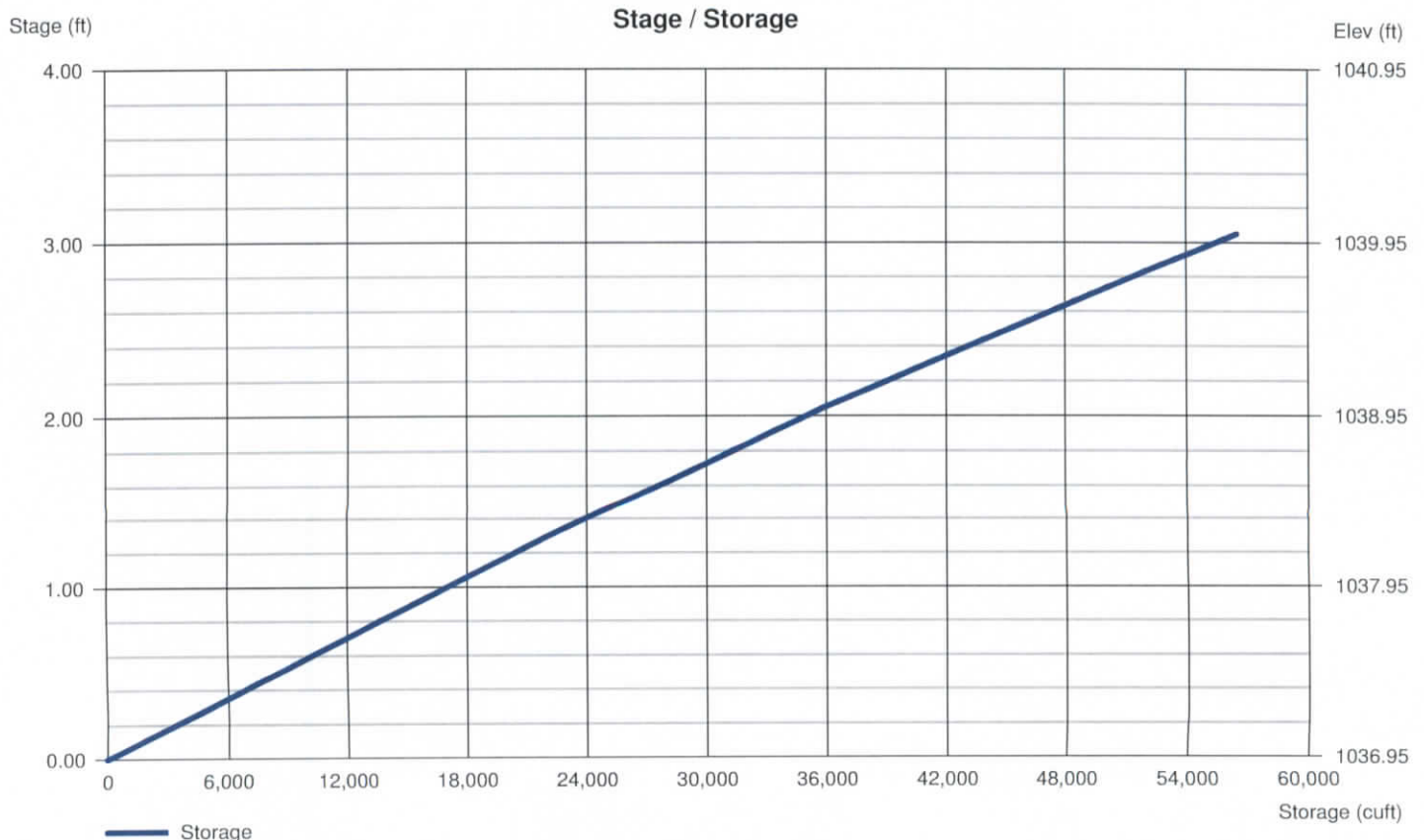
### Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 0.00	0.00	0.00	0.00
Span (in)	= 0.00	0.00	0.00	0.00
No. Barrels	= 0	0	0	0
Invert El. (ft)	= 0.00	0.00	0.00	0.00
Length (ft)	= 0.00	0.00	0.00	0.00
Slope (%)	= 0.00	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

### Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 1.50	5.00	5.00	15.00
Crest El. (ft)	= 1036.95	1037.95	1037.95	1038.95
Weir Coeff.	= 3.33	3.33	3.33	2.60
Weir Type	= Rect	Rect	Rect	Broad
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



STAGE - STORAGE CURVE (detention volume)					
WQ VOL - POND A					
ELEVATION	STAGE	AREA	AVE. AREA	VOLUME	TOTAL VOLUME
		FT^2			cu. Ft
<b>1,031.70</b>	0.00	0.00		0	<b>0</b>
	-	-			-
	-	-	138.50		-
<b>1,032.00</b>	0.30	277.00		42	<b>42</b>
	-	-			-
	-	-	3,394.50		-
<b>1,032.40</b>	0.70	6,512.00		1,358	<b>1,399</b>
	-	-			-
	-	-	8,044.50		-
<b>1,034.00</b>	2.30	9,577.00		12,871	<b>14,271</b>
	-	-			-
	-	-	11,069.50		-
<b>1,035.86</b>	4.16	12,562.00		20,589	<b>34,860</b>
	-	-			-
	-	-	14,504.50		-
<b>1,036.00</b>	4.30	16,447.00		2,031	<b>36,890</b>
	-	-			-
	-	-	16,898.50		-
<b>1,036.95</b>	5.25	17,350.00		16,054	<b>52,944</b>



## Pond B - As-Built Summary

Pond B				Top of Berm: 1013.4	
Design Storm	As-Designed Release Rates (cfs)	As-Built Release Rates (cfs)	As-Designed Water Surface Elevation (ft)	As-Built Water Surface Elevation (ft)	As-Built Freeboard Provided (ft)
1	1.72	1.80	1008.78	1008.57	4.83
2	2.42	2.52	1008.98	1008.77	4.63
5	4.56	4.75	1009.38	1009.18	4.22
10	6.83	7.01	1009.67	1009.48	3.92
25	10.43	10.73	1010.05	1009.86	3.54
50	13.40	13.61	1010.33	1010.12	3.28
100	15.46	15.55	1010.51	1010.29	3.11

Basin B - Volume Summary						
Design Storm	Water Quality Volume (cf)	Channel Protection Volume	Diameter of Water Quality Orifice (ft)	Elevation of Water Quality Orifice (ft)	Diameter of Channel Protection Orifice (ft)	Elevation of Channel Protection Orifice (ft)
As-Designed	79,315	N/A	N/A	1008.00	N/A	N/A
As-Built	77,094	N/A	N/A	1007.77	N/A	N/A

Note: Pond B is a wet pond with the water quality volume completely contained in the permanent pool thus no draw down orifice is provided for Pond B. The calculated water quality volume required for the entire development is 16,195 cf as per the approved hydrology study by David A. Burre Engineers & Surveyors most recently revised March 1, 2007. The the cumulative as-built water quality volume provided in both Pond A & B is 103,038 cf.

Additionally, Crystal Streams units are provided upstream of each of the outfalls into Pond B. (four units total). Though these units were installed as pre-treatment for Pond B the site will receive added water quality benefit in addition to the treatment received by the wet ponds.

Note: Channel Protection is not provided in Pond B as it was not included in the approved hydrologic design noted above. Thus, channel protection volume and orifice information is not included in the summary table.

Note: A plunge pool was installed at the outfall of Pond B under the previous permit for Old Alabama Townhomes. This plunge pool has remained intact though it was unmaintained while the project set dormant for over 5 years. The plunge pool is to remain in place and will provide energy dissipation and slowing discharge velocities.

Basin B - Flow Summary						
Design Storm	Predeveloped Flow (cfs)	As-Built Flow to Pond (cfs)	As-Built Bypass (cfs)	Allowable Flow* (cfs)	As-Built Pond Flow (cfs)	Flow Reduction (cfs)
1	2.3	14.8	0.5	1.8	1.8	0.0
2	3.7	18.6	0.7	3.0	2.5	0.4
5	7.0	26.4	1.4	5.6	4.8	0.8
10	9.8	32.4	2.0	7.8	7.0	0.8
25	13.8	40.5	2.8	11.0	10.7	0.3
50	17.2	46.7	3.5	13.7	13.6	0.1
100	19.6	50.8	4.0	15.7	15.6	0.1

\*Allowable Flow = Predeveloped Flow less As-Built Bypass

Allowable Flow is the basis of comparison for the pond release rates as per the approved hydrology study. The Flow Summary table format was taken from the originally approved Hydrology Study noted above.

STAGE - STORAGE CURVE (detention volume)					
WQ VOL - POND B					
ELEVATION	STAGE	AREA	AVE. AREA	VOLUME	TOTAL VOLUME
		FT^2			cu. Ft
<b>1,004.15</b>	0.00	16,369.00		0	<b>0</b>
	-	-			-
	-	-	18,823.00		-
<b>1,006.00</b>	1.85	21,277.00		34,823	<b>34,823</b>
	-	-			-
	-	-	22,195.50		-
<b>1,006.84</b>	2.69	23,114.00		18,644	<b>53,467</b>
	-	-			-
	-	-	25,406.00		-
<b>1,007.77</b>	3.62	27,698.00		23,628	<b>77,094</b>

# Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Tuesday, Jan 7, 2014

## Pond No. 2 - Detention Pond B

### Pond Data

Contours - User-defined contour areas. Average end area method used for volume calculation. Beginning Elevation = 1007.77 ft

### Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	1007.77	27,783	0	0
2.20	1009.97	27,783	61,123	61,123
3.23	1011.00	33,330	31,473	92,596
4.23	1012.00	39,654	36,492	129,088

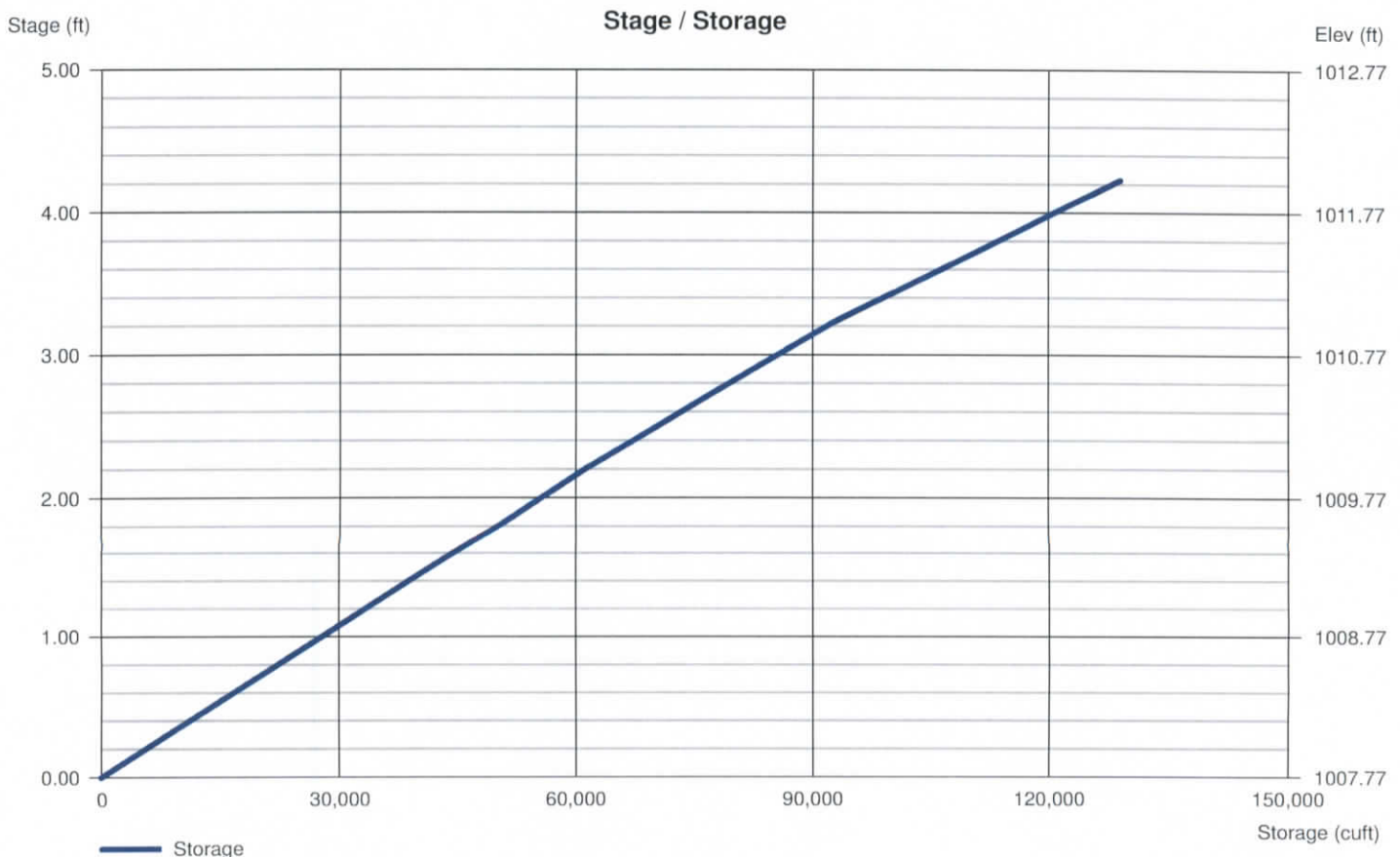
### Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 0.00	0.00	0.00	0.00
Span (in)	= 0.00	0.00	0.00	0.00
No. Barrels	= 0	0	0	0
Invert El. (ft)	= 0.00	0.00	0.00	0.00
Length (ft)	= 0.00	0.00	0.00	0.00
Slope (%)	= 0.00	0.00	0.00	n/a
N-Value	= .000	.000	.000	n/a
Orifice Coeff.	= 0.00	0.00	0.00	0.00
Multi-Stage	= n/a	No	No	No

### Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 0.75	1.00	12.00	0.00
Crest El. (ft)	= 1007.77	1008.87	1010.95	0.00
Weir Coeff.	= 3.33	3.33	2.60	0.00
Weir Type	= Rect	Rect	Broad	---
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



## **1, 2, 5, 10, 25, 50, 100-YEAR SUMMARY REPORTS**

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## 100 – YEAR HYDROGRAPHS

# Hydrograph Report

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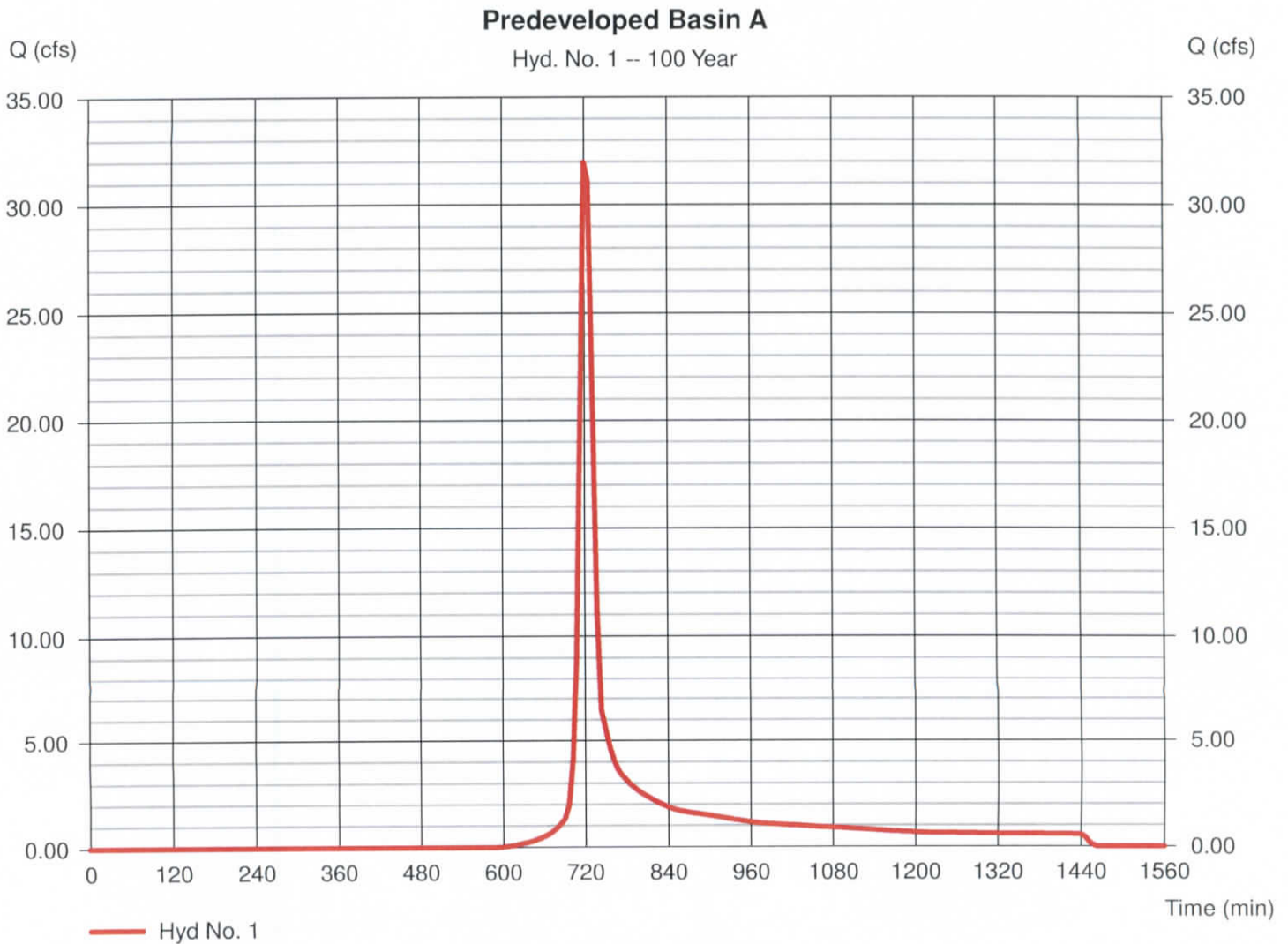
Tuesday, Jan 7, 2014

## Hyd. No. 1

### Predeveloped Basin A

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 6 min  
Drainage area = 9.480 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 31.98 cfs  
Time to peak = 720 min  
Hyd. volume = 101,588 cuft  
Curve number = 60.5  
Hydraulic length = 705 ft  
Time of conc. (Tc) = 14.80 min  
Distribution = Type II  
Shape factor = 484



# Hydrograph Report

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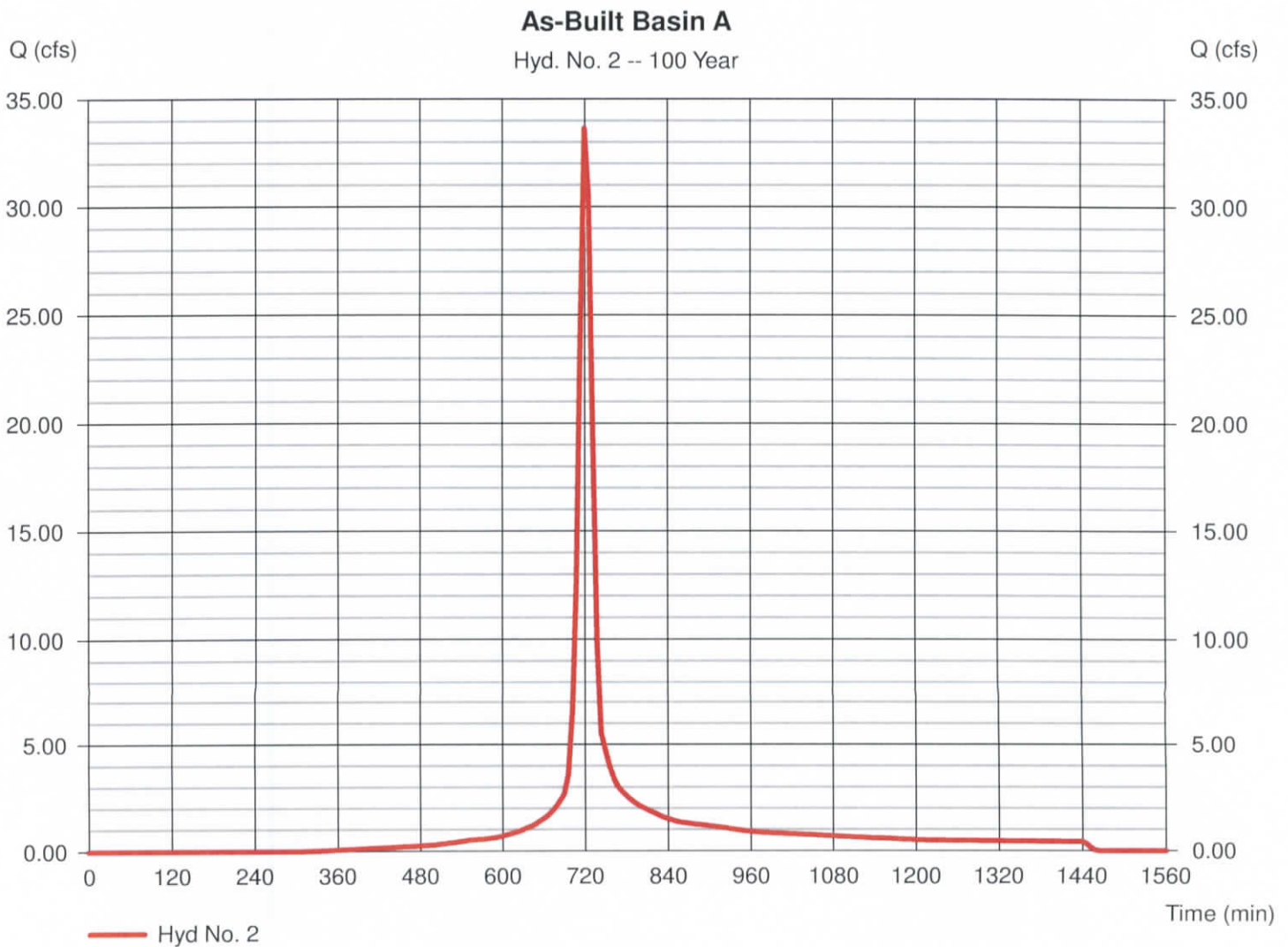
Tuesday, Jan 7, 2014

## Hyd. No. 2

### As-Built Basin A

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 6 min  
Drainage area = 5.630 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 33.65 cfs  
Time to peak = 720 min  
Hyd. volume = 104,246 cuft  
Curve number = 81  
Hydraulic length = 980 ft  
Time of conc. (Tc) = 10.98 min  
Distribution = Type II  
Shape factor = 484





# Hydrograph Report

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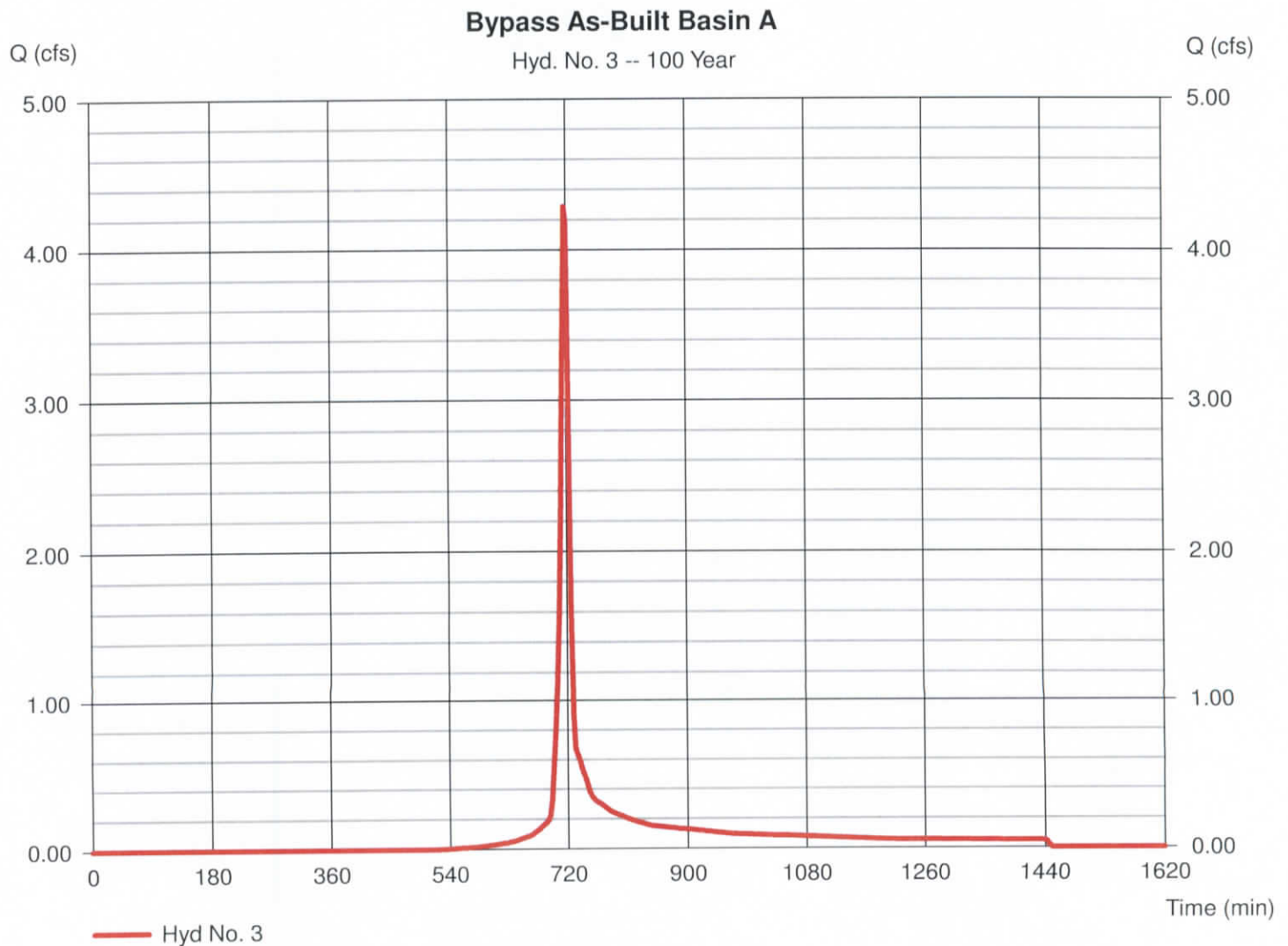
Tuesday, Jan 7, 2014

## Hyd. No. 3

### Bypass As-Built Basin A

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 3 min  
Drainage area = 0.760 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 4.290 cfs  
Time to peak = 717 min  
Hyd. volume = 9,691 cuft  
Curve number = 66  
Hydraulic length = 350 ft  
Time of conc. (Tc) = 7.38 min  
Distribution = Type II  
Shape factor = 484



# Hydrograph Report

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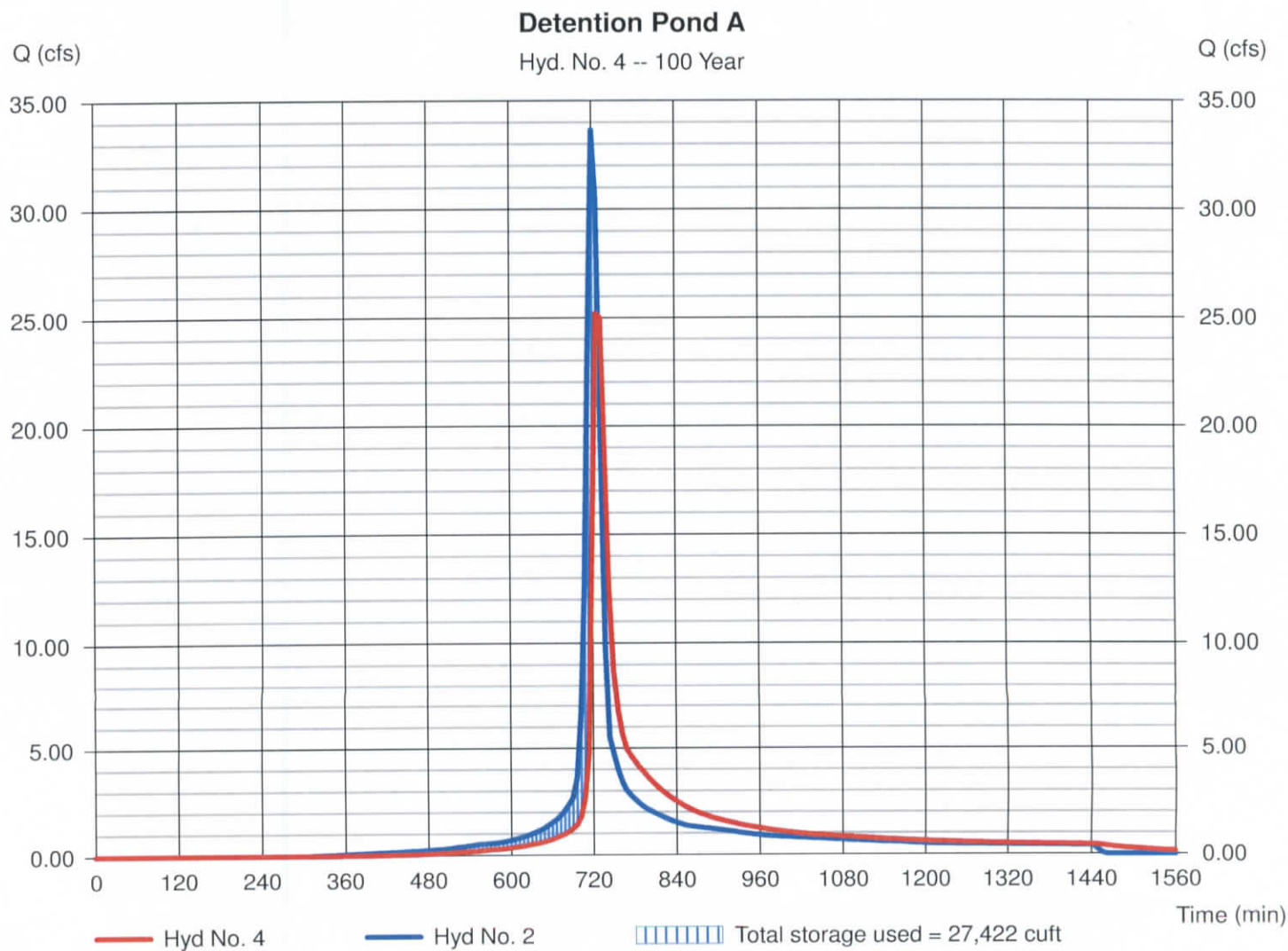
## Hyd. No. 4

### Detention Pond A

Hydrograph type = Reservoir  
Storm frequency = 100 yrs  
Time interval = 6 min  
Inflow hyd. No. = 2 - As-Built Basin A  
Reservoir name = Detention Pond A

Peak discharge = 25.20 cfs  
Time to peak = 726 min  
Hyd. volume = 104,232 cuft  
Max. Elevation = 1038.59 ft  
Max. Storage = 27,422 cuft

Storage Indication method used.



# Hydrograph Report

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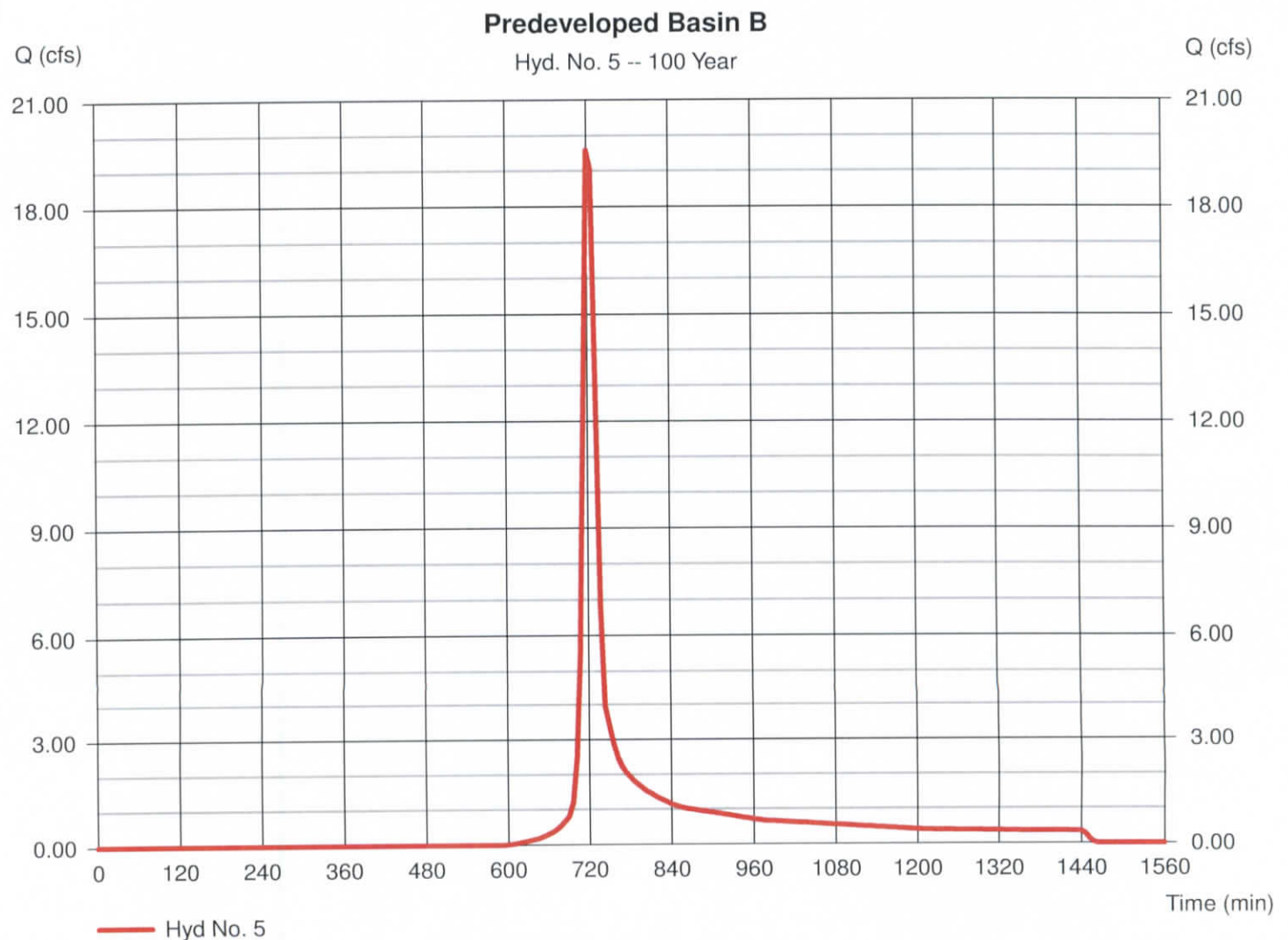
Tuesday, Jan 7, 2014

## Hyd. No. 5

### Predeveloped Basin B

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 6 min  
Drainage area = 5.810 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 19.60 cfs  
Time to peak = 720 min  
Hyd. volume = 62,260 cuft  
Curve number = 60.5  
Hydraulic length = 885 ft  
Time of conc. (Tc) = 17.80 min  
Distribution = Type II  
Shape factor = 484



# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

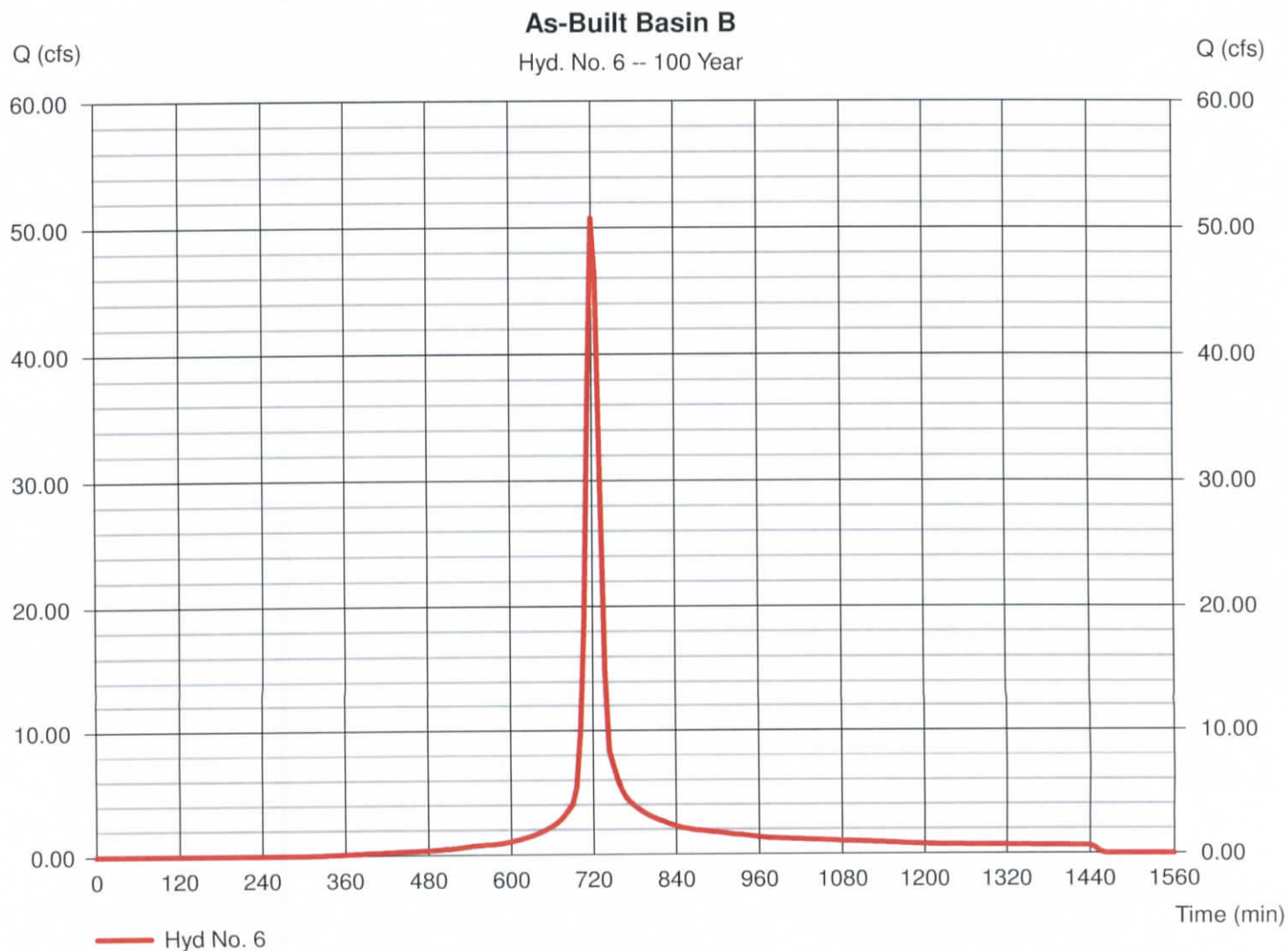
Tuesday, Jan 7, 2014

## Hyd. No. 6

### As-Built Basin B

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 6 min  
Drainage area = 8.510 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 50.76 cfs  
Time to peak = 720 min  
Hyd. volume = 157,239 cuft  
Curve number = 80.9  
Hydraulic length = 1100 ft  
Time of conc. (Tc) = 12.08 min  
Distribution = Type II  
Shape factor = 484



# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

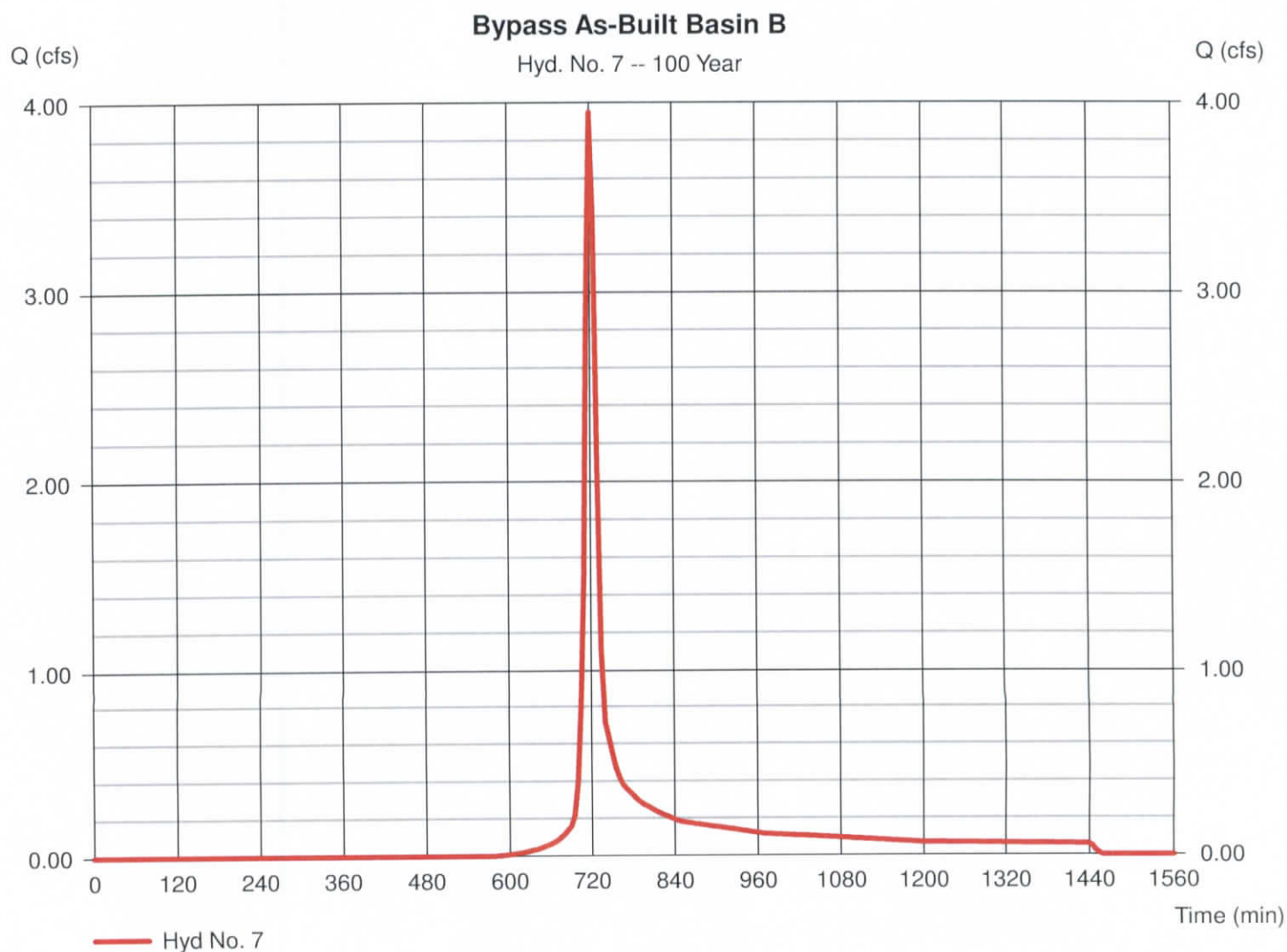
Tuesday, Jan 7, 2014

## Hyd. No. 7

### Bypass As-Built Basin B

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 5 min  
Drainage area = 0.970 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 3.948 cfs  
Time to peak = 720 min  
Hyd. volume = 10,785 cuft  
Curve number = 61.6  
Hydraulic length = 570 ft  
Time of conc. (Tc) = 12.21 min  
Distribution = Type II  
Shape factor = 484





# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Tuesday, Jan 7, 2014

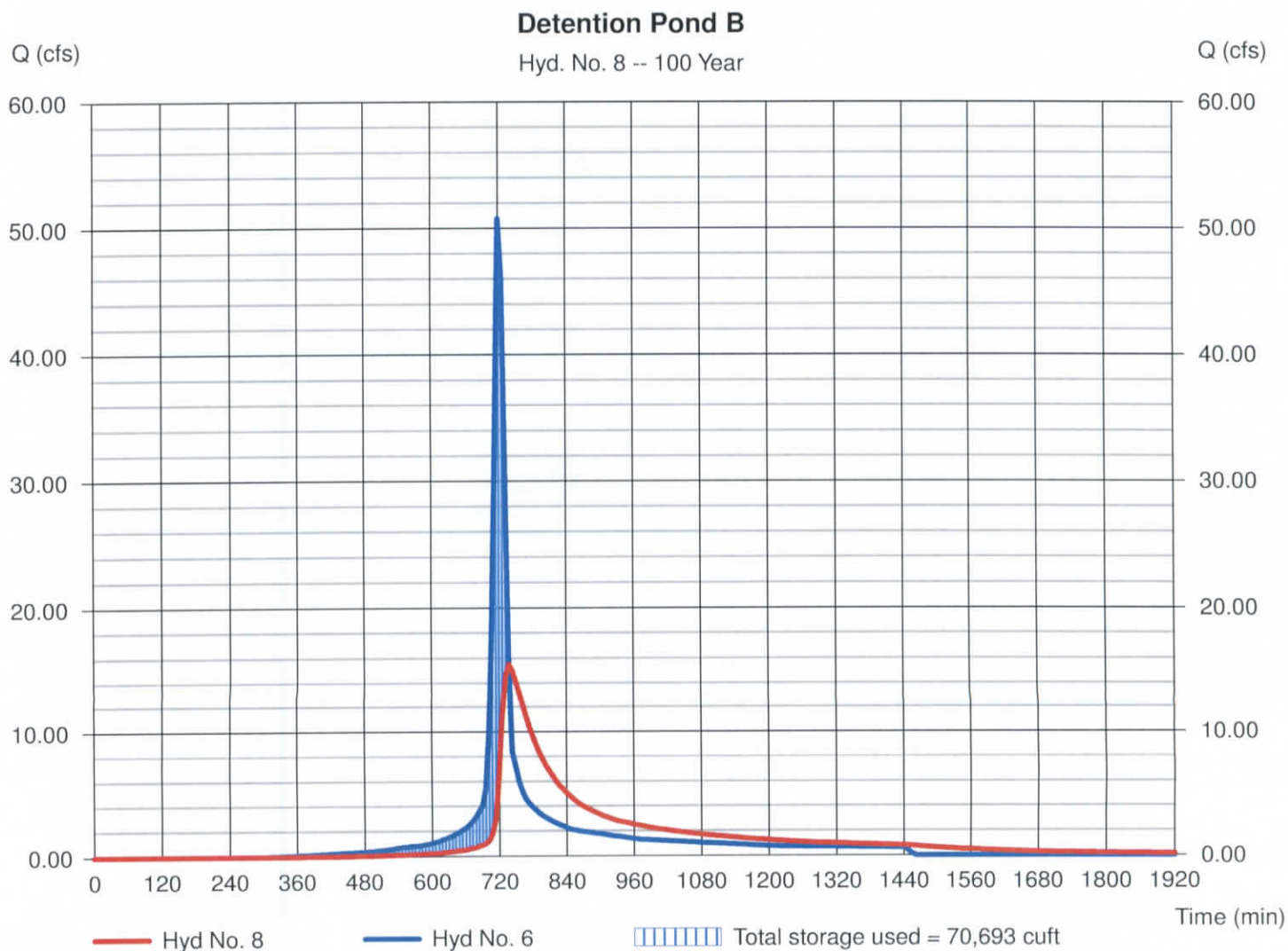
## Hyd. No. 8

Detention Pond B

Hydrograph type = Reservoir  
Storm frequency = 100 yrs  
Time interval = 6 min  
Inflow hyd. No. = 6 - As-Built Basin B  
Reservoir name = Detention Pond B

Peak discharge = 15.55 cfs  
Time to peak = 738 min  
Hyd. volume = 157,215 cuft  
Max. Elevation = 1010.29 ft  
Max. Storage = 70,693 cuft

Storage Indication method used.



# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

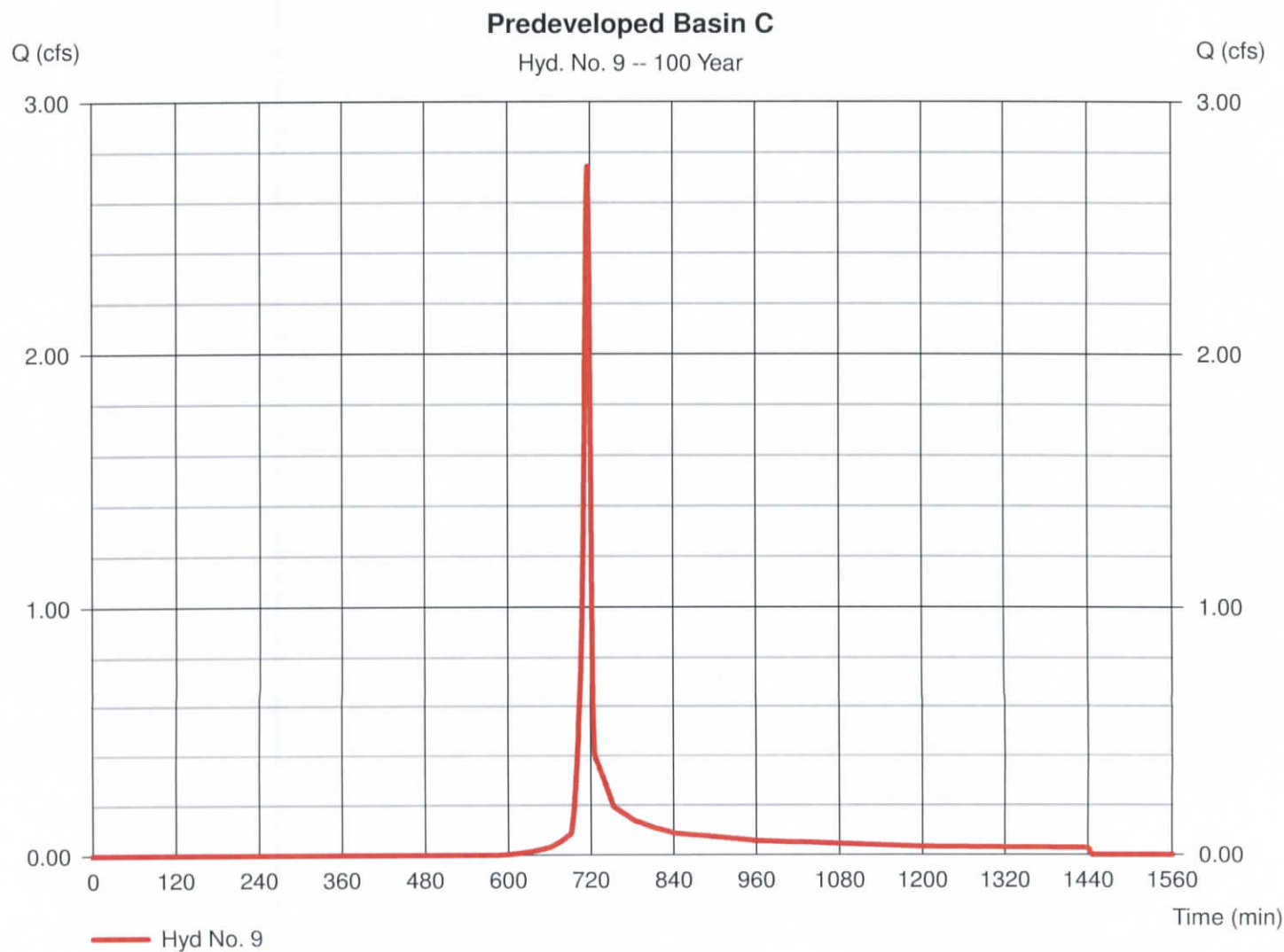
Tuesday, Jan 7, 2014

## Hyd. No. 9

### Predeveloped Basin C

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 1 min  
Drainage area = 0.450 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 2.745 cfs  
Time to peak = 717 min  
Hyd. volume = 5,144 cuft  
Curve number = 60.5  
Hydraulic length = 120 ft  
Time of conc. (Tc) = 3.60 min  
Distribution = Type II  
Shape factor = 484



# Hydrograph Report

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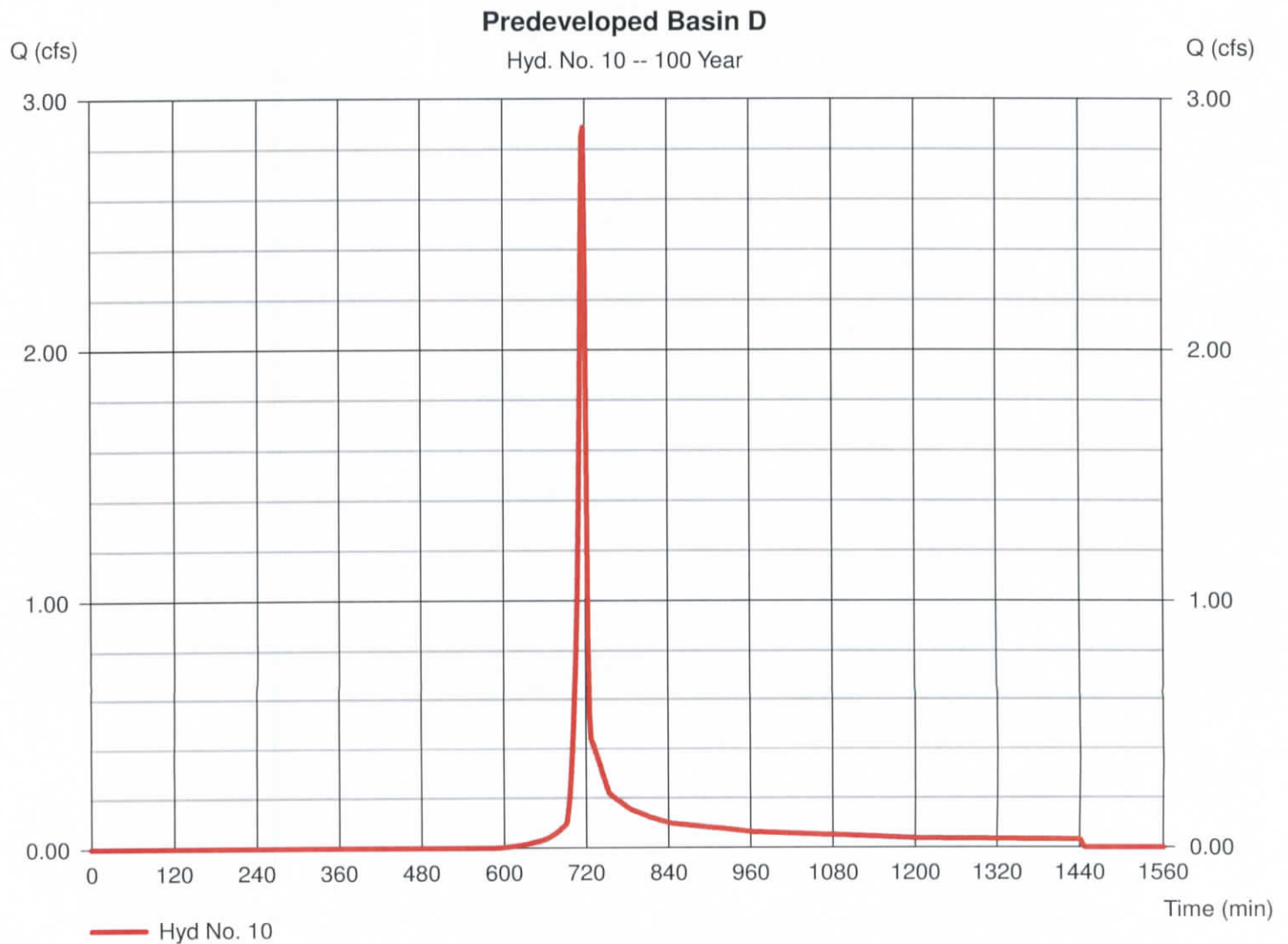
Tuesday, Jan 7, 2014

## Hyd. No. 10

### Predeveloped Basin D

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 2 min  
Drainage area = 0.540 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 2.887 cfs  
Time to peak = 718 min  
Hyd. volume = 5,787 cuft  
Curve number = 60.5  
Hydraulic length = 150 ft  
Time of conc. (Tc) = 4.30 min  
Distribution = Type II  
Shape factor = 484



# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

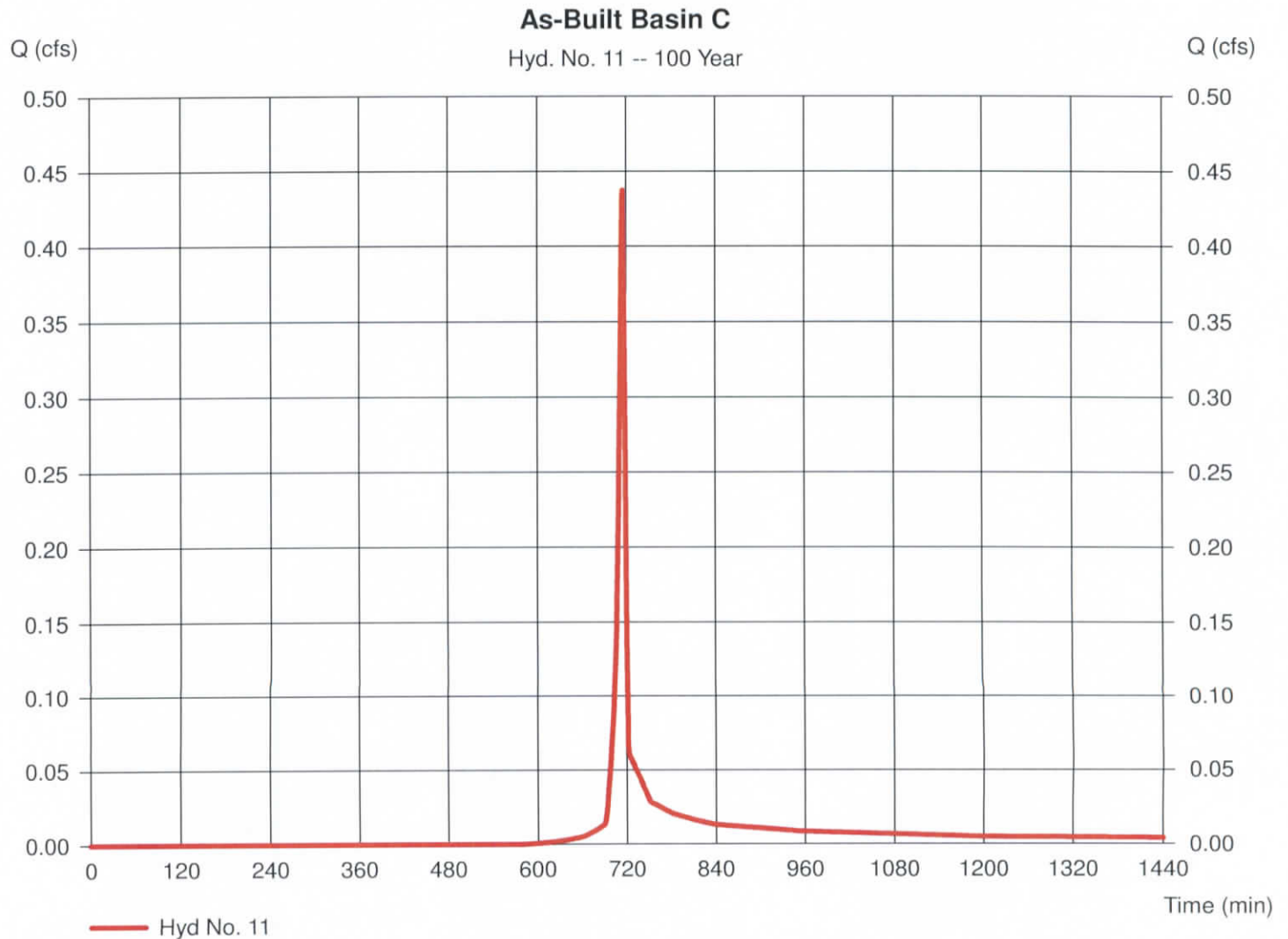
Tuesday, Jan 7, 2014

## Hyd. No. 11

### As-Built Basin C

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 1 min  
Drainage area = 0.070 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 0.437 cfs  
Time to peak = 716 min  
Hyd. volume = 763 cuft  
Curve number = 61  
Hydraulic length = 80 ft  
Time of conc. (Tc) = 2.58 min  
Distribution = Type II  
Shape factor = 484



# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

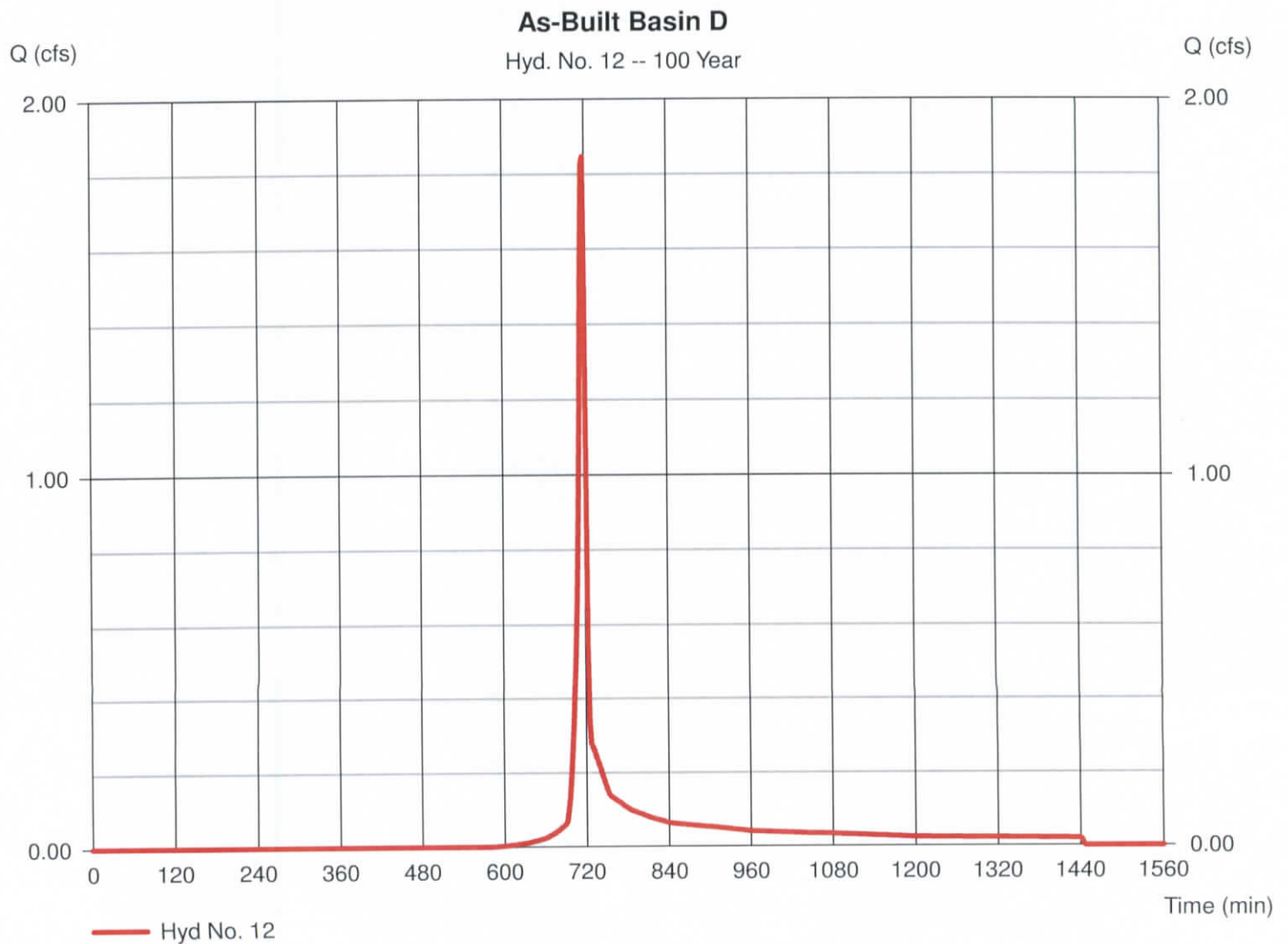
Tuesday, Jan 7, 2014

## Hyd. No. 12

### As-Built Basin D

Hydrograph type = SCS Runoff  
Storm frequency = 100 yrs  
Time interval = 2 min  
Drainage area = 0.340 ac  
Basin Slope = 7.6 %  
Tc method = LAG  
Total precip. = 7.68 in  
Storm duration = 24 hrs

Peak discharge = 1.848 cfs  
Time to peak = 718 min  
Hyd. volume = 3,706 cuft  
Curve number = 61  
Hydraulic length = 150 ft  
Time of conc. (Tc) = 4.26 min  
Distribution = Type II  
Shape factor = 484



## APPENDIX



Project:  
Location: Basin A  
Developed

By:  
Checked:  
Date: 01/06/14  
Date:

1. Runoff Curve Number (CN)

Cover description	CN	Soil Group	Area (Acre)
Impervious- Misc	98		0.306
Impervious-Driveway	98		0.284
Impervious-House	98		1.114
Impervious-Roadway	98		0.790
Impervious-Sidewalk	98		0.142
Landscaped	61		2.580
Water Surface	98		0.414

CN (weighted): 81.0  
Total Area: 5.630 Acre

2. Runoff

Return Period: 1-Year  
Rainfall, P: 0.00 in  
Runoff, Q: 0.0000 in  
Runoff Volume: 0.0000 Acre-Ft

Project:  
Location: Basin A-Bypass  
Developed

By:  
Checked: Date: 01/06/14  
Date:

## 1. Runoff Curve Number (CN)

Cover description	CN	Soil Group	Area (Acre)
Impervious-House	98		0.103
Landscaped	61		0.659

CN (weighted): 66.0  
Total Area: 0.762 Acre

## 2. Runoff

Return Period:	1-Year	
Rainfall, P:	0.00	in
Runoff, Q:	0.0000	in
Runoff Volume:	0.0000	Acre-Ft

Project:  
Location: Basin B  
Developed

By:  
Checked: Date: 01/07/14  
Date:

1. Runoff Curve Number (CN)

Cover description	CN	Soil Group	Area(Acre)
Impervious- Misc	98		0.125
Impervious-Driveway	98		0.555
Impervious-House	98		2.230
Impervious-Roadway	98		0.857
Impervious-Sidewalk	98		0.166
Landscaped	61		3.923
Water Surface	98		0.651

CN (weighted): 80.9  
Total Area: 8.507 Acre

2. Runoff

Return Period:	1-Year	
Rainfall, P:	0.00	in
Runoff, Q:	0.0000	in
Runoff Volume:	0.0000	Acre-Ft

Date: 01/07/14  
Date:

Landscaped

CN	Soil Group	Area (Acre)
98		0.016
61		0.958

Runoff Volume:

```
1-Year
0.00      in
0.0000    in
0.0000    Acre-Ft
```